# National Exams December 2016 

## 04-BS-6: Mechanics of Materials

3 hours duration

Notes:

1. If doubt exists as to the interpretation of any question, the candidate is urged to submit with the answer paper a clear statement of any assumptions made.
2. Candidates may use one of two calculators, the Casio or Sharp approved models.

This is a Closed Book exam. However candidates are permitted to bring the following into the examination room:

- ONE aid sheet $8.5^{\prime \prime} \times 11^{\prime \prime}$ hand-written on both sides containing notes and formulae. Example problems and solutions to problems are not allowed!

3. Any FIVE (5) questions (out of 8 given) constitute a complete paper. Only the first five questions as they appear in your answer book will be marked.
4. All questions are of equal value.
5. Information on geometric properties of wide flange or W shape sections is attached at the end of this exam. Note that this information may not be required.

## NOTE: The aid sheet must be handed in with the exam!

Your exam will not be marked if you do not hand in an aid sheet, unless there is a signed statement by the exam invigilator stating that no aid sheet was used for the exam.

Question 1: A simply supported beam supports a triangularly distributed load and a couple acting [20 marks] at the left support as shown. The beam has the cross-section given and is made of steel with a yield strength of 350 MPa and shear stress at yield of 75 MPa . The elastic modulus of the steel is 200 GPa .

Determine the SHEAR FORCE and BENDING MOMENT along the length of the beam as a function of $x$. In other words, find $V(x)$ and $M(x)$ for the beam.

Then draw the corresponding shear force and bending moment diagrams for the beam (label all critical points and show your work by indicating exactly how you obtained your answers).

beam cross-section (all dimensions in mm)

Question 2: A simply supported beam supports a triangularly distributed load and a couple [20 marks] acting at the left support as shown. The beam has the cross-section given and is made of steel with a yield strength of 350 MPa and shear stress at yield of 75 MPa . The elastic modulus of the steel is 200 GPa .
(a) Determine the maximum deflection using the method of integration.
(b) Determine the slope at the left support using the method of integration.
(c) Sketch the deflected shape of the beam and indicate whether the beam satisfies an allowable deflection limit of $\mathrm{L} / 120$ (where L equals the span of the beam).


Question 3: A simply supported beam supports a triangularly distributed load and a couple [20 marks] acting at the left support as shown. The beam has the cross-section given and is made of steel with a yield strength of 350 MPa and shear stress at yield of 75 MPa . The elastic modulus of the steel is 200 GPa .

Determine the following:
(a) maximum normal stress in the beam
(b) maximum shear stress in the beam
(c) shear stress at the tip of the flange (point E) at a section located 2 m from the left support. To receive marks you must give reasons to justify your answer.


Question 4: A steel plate $100 \mathrm{~mm} \times 100 \mathrm{~mm}$ in plan and 2 mm thick is subjected to the axial and shear forces as shown. Ignore stress concentrations from the forces.
[20 marks] Use the Mohr's circle solution (not the transformation equations) to determine the following:
(a) the principal stresses and orientation of the principal planes, showing your answer on a properly oriented element.
(b) the maximum in-plane shear stress (and associated normal stresses) and orientation of the corresponding planes. Once again, show your answer on a sketch of a properly oriented element.


Careful: Mohr's circle involves stresses .... Not forces.
WARNING! Credit will only be given for a solution using Mohr's circle. Not the stress transformation equations. This means that you need to draw a Mohr's circle based on the stress components given in this problem. Remember to show numbers on your circle. Your calculations must be based on the geometry of your circle. So use your calculator. In other words, you are expected to use trigonometry to construct your Mohr's circle. Do not give a graphical solution that is scaled off!

The stress transformation equations can only be used to check your answer.

Question 5: A uniformly distributed load of $40 \mathrm{kN} / \mathrm{m}$ is applied to a horizontal beam AB that is [20 marks] supported by a pinned connection at A and an inclined tension member at B . The beam has the cross section shown and is made of steel with a normal yield stress of 350 MPa and yield stress in shear of 60 MPa . The elastic modulus of the steel equals 200 GPa . The pinned connections at A and B are located at the centroid of the beam cross-section.
(a) Compute the distribution of normal stress in the I-beam at a section located 1 m from the support at $A$. Show this distribution on a sketch and make sure to show maximum and minimum values of stress.
(b) Compute the maximum shear stress in the I-beam at the same section located 1 m from the support at $A$.
(c) What happens when the pinned connections at A and B are not located at the centroid of the beam cross-section.

beam cross-section (all dimensions in mm )

Question 6: A stepped steel shaft with $\mathrm{G}=80 \mathrm{GPa}$ and $\tau_{y}=250 \mathrm{MPa}$ is subjected to the [20 marks] torques shown (note that one of the torques is a distributed load). Dimensions (length and diameter) are also given.
(a) Determine the maximum shear stress in the stepped shaft, and sketch the corresponding variation of shear stress along the shaft radius at this location.
(b) Determine the rotation (in degrees) at the free end of the shaft.


Question 7: A rigid L -shaped link ( ABCDE ) is supported by a 12 mm diameter pin at D and [20 marks] two 3 mm diameter cables at points $B$ and $C$. Both cables have a length of 1000 mm and are made of steel with a yield strength of 400 MPa and elastic modulus of 200 GPa . The L-shaped link is loaded with a concentrated load at E as shown.
(a) find the forces developed in each cable
(b) find the corresponding horizontal displacement at the top of the link (point A)
(c) find the shear stress in the pin at D given that the pin is loaded in double shear.


Question 8 Determine the largest load $P$ that can be applied to the truss structure below given [20 marks] that members $A B$ and $B C$ are made of 120 mm outside diameter hollow steel pipes with a 10 mm wall thickness. Both steel pipes are pinned at their ends.

Consider in-plane buckling only for the compression members and use a factor of safety of 2 for the Euler buckling load. Do not use a safety factor for yielding of the steel. The steel used in the rods has an allowable yield strength equal to 240 MPa and elastic modulus of 200 GPa .


Note: $\quad \mathrm{A}_{\text {circle }}=\pi \mathrm{r}^{2}$ and $\mathrm{I}_{\text {circle }}=\pi \mathrm{r}^{4} / 4$


|  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Designation | Area $A$ | Depth d | Web thickness $t_{w}$ | Flange |  | $x-x$ axis |  |  | $y \mathrm{y}$ y x xim |  |  |
|  |  |  |  | width | thickness |  |  |  |  |  |  |
|  |  |  |  | $\mathrm{b}_{5}$ | \% | 1 | 5 | $r$ | , | 5 | r |
| mmxkg/m | $\mathrm{mm}^{2}$ | mm | mm | mm | mm | $10 . \mathrm{mm}^{2}$ | $10^{3} \mathrm{~mm}^{3}$ | mm | 10 mma | $10^{2} \mathrm{~mm}^{3}$ | mm |
| W610 $\times 155$ | 19800 | 611 | 12.70 | 324.0 | 19.0 | 1290 | 4220 | 255 | 108 | 607 | 73.9 |
| W610 $\times 140$ | 1790 | 617 | 13.10 | 230.0 | 22.2 | 1120 | 3630 | 250 | 45.1 | 392 | 502 |
| W610 $\times 125$ | 1590 | 612 | 11.90 | 229.0 | 19.6 | 985 | 3220 | 249 | 34.3 | 3.43 | 40.7 |
| 13610 1103 | 14.400 | 608 | 11.20 | 228.0 | 17.3 | 875 | 2880 | 248 | 34.3 | 301 | 48.8 |
| W6to $\times 100$ | 12900 | 603 | 10.50 | 228.0 | 14.9 | 764 | 2530 | 24.3 | 29.5 | 259 | 47.8 |
| W610 $\times 92$ | 11800 | 603 | 10.90 | 179.0 | 15.0 | 646 | 2140 | 234 | 14.4 | 16. | 34.9 |
| Whto 882 | 10.500 | 599 | 10.00 | 178.0 | 12.8 | 560 | 1870 | 231 | 12.1 | 136 | 339 |
| W460 $\times 0$ | 12300 | 466 | 11.40 | 193.0 | 19.0 | 445 | 1910 | 190 | 228 | 236 | 43.1 |
| W460 $\times 89$ | 11.40) | 463 | 10.50 | 192.0 | 17.7 | 410 | 1770 | 190 | 20.9 | 218 | 42.8 |
| W460 $\times 8.2$ | 10400 | 460 | 9.91 | 191.0 | 16.0 | 370 | 1610 | 189 | 18.6 | 195 | 42.3 |
| W460 1.74 | 9460 | 457 | 9.02 | 1900 | 14.5 | 337 | 1460 | 188 | 16.6 | 175 | 41.9 |
| $14.60 \times 68$ | 8730 | 459 | 9.14 | 1.4 .0 | 15.4 | 297 | 1290) | 184 | 9.41 | 122 | 32.8 |
| W460 $\times 60$ | 7590 | 455 | 8.60 | 153.0 | 13.3 | 255 | 1120 | 183 | 7.96 | 104 | 32.4 |
| W460 $\times 52$ | 6640 | 450 | 7.62 | 152.0 | 10.8 | 212 | 942 | 179 | 6.34 | 83.4 | 30.9 |
| $W 410 \times 85$ | 10800 | 417 | 10.90 | 181.0 | 18.2 | 315 | $151 \%$ | 171 | 18.0 | 199 | 40.8 |
| W410 $\times 74$ | 9510 | 413 | 9.65 | 1500 | 16.0 | 275 | 1330 | 170 | 156 | 173 | 40.5 |
| W410 $\times 67$ | 8560 | 410 | 8.76 | 179.0 | 14.4 | 245 | 1206 | 169 | 13.8 | 154 | 40.2 |
| W410 $\times 53$ | 6820 | 403 | 7.49 | 177.0 | 10.9 | 186 | 923 | 165 | 10.1 | 114 | 38.5 |
| $W 410 \times 16$ | 5890 | 403 | 6.99 | 140.0 | 11.2 | 156 | 774 | 163 | 5.14 | 73.4 | 29.5 |
| W410 $\times 39$ | 4960 | 399 | 6.35 | 140.0 | 8.8 | 126 | 632 | 159 | 4,02 | 57.4 | 28.5 |
| W360 $\times 79$ | 10100 | 354 | 9.40 | 205.0 | 16.8 | 227 | 1280 | 150 | 24.2 | 236 | 48.9 |
| W $360 \times 64$ | 8150 | 347 | 7.75 | 203.0 | 13.5 | 179 | 1090 | 148 | 18.6 | 18.5 | 48.0 |
| W360 $\times 9$ | 7200 | 358 | 7.87 | 172.0 | 12.8 | $16 \%$ | 894 | 149 | 11.1 | 129 | 34.3 |
| W360 $\times 51$ | 6450 | 355 | 7.24 | 171.0 | 11.6 | 141 | 794 | 14 | 9.0s | 11.3 | 38.7 |
| W360 $\times 45$ | 5710 | 352 | 0.86 | 171.0 | 4.8 | 121 | $68 \%$ | 1.46 | 8.16 | 95.4 | 37.8 |
| W $360 \times 39$ | 4960 | 353 | 6.48 | 188.0 | 10.7 | 102 | 578 | 143 | 3.75 | 58.6 | 27.5 |
| W360 $\times 33$. | 4190 | 3.49 | 5.8 .4 | 127.0 | 8.5 | 82.9 | 475 | 141 | 3.91 | 458 | 26.4 |




| Designation | $\begin{gathered} \text { Area } \\ \text { A } \end{gathered}$ | Depth d | Web thickness $t_{\text {sk }}$ | Flange |  | $x-x$ axis |  |  | $y$-yaxis |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | width b: | thickness $t$ |  |  |  |  |  |  |
|  |  |  |  |  |  | 1 | 5 | $r$ | 1 | 5 | ' |
| $m m \times \mathrm{kg} / \mathrm{m}$ | $\mathrm{mm}^{2}$ | mm | mm | mm | mm | $10^{5} \mathrm{~mm}^{4}$ | $10^{3} \mathrm{~mm}^{3}$ | mm | $10^{6} \mathrm{~mm}^{4}$ | $10^{3} \mathrm{~mm}^{3}$ | mm |
| W310 $\times 129$ | 16.500 | 318 | 13.10 | 308.0 | 20.6 | 308 | 1940 | 137 | 100 | 6.49 | 77.8 |
| W310 $\times 7.4$ | 9.480 | 316 | 9.40 | 2050 | 16.3 | 165 | 1060 | 132 | 23.4 | 228 | 49.7 |
| W310 $\times 67$ | 8530 | 36 | 8.51 | 2940 | 14.6 | 1.5 | 948 | 13n | 20.7 | 203 | 49.3 |
| W310 $\times 39$ | 4930 | 310 | 5.84 | 1650 | 9.7 | Sts | 547 | 131 | 7.23 | 87.6 | 38.3 |
| W310 $\times 3.3$ | 4189 | 13 | 6.60) | 102.0 | 10.8 | 650 | 415 | 125 | 1.92 | 37.6 | 21.4 |
| W310 $\times 24$ | 3040 | 305 | 5.59 | 101.9 | (1, \% | 428 | 2 St | 119 | 1.16 | 23.0 | 10.5 |
| W310 $\times 21$ | 2600 | Wh | 508 | 1040 | 5.7 | 376 | 24. | 117 | 0086 | 19.5 | 19.2 |
| W250 $\times 149$ | 19000 | 282 | 17.30 | 26.30 | 28.4 | 259 | 18.46 | 117 | 80.2 | 656 | 67.4 |
| W250 $\times 80$ | 10200 | 256 | 9.40 | 2550 | 15.6 | 26 | 984 | 111 | 43.1 | 338 | 650 |
| W250 $\times 67$ | 8560 | 257 | 8.89 | 314.0 | 15.7 | 164 | *i9\% | 110 | 222 | 218 | 50.9 |
| W250 $\times 58$ | 7400 | 252 | sum | 36.0 | 13.5 | 87.3 | 69 | 104 | 18.8 | 185 | 50,4 |
| W250 $\times 45$ | 5700 | 266 | 7.62 | 148.0 | 13.0 | 7.1 | 536 | 112 | 7.03 | 95 | 35.1 |
| W250 228 | 3620 | 260 | 6.35 | 102.0 | 10.0 | 39.9 | 307 | 10.5 | 1.78 | 34.9 | 22.2 |
| W250 $\times 22$ | 2850 | 254 | 5.84 | 102.0 | 6.9 | 28.8 | 227 | 101 | 1.22 | 23.9 | 207 |
| W250 $\times 18$ | 2280 | 251 | 4.83 | 10.0 | 53 | 22.5 | 179 | 99.3 | 0.919 | 18.2 | 20.1 |
| W200 $\times 100$ | 12700 | 224 | 14.50 | 2100 | 23.7 | 113 | 987 | 94.3 | 36.6 | 349 | 53.7 |
| W200 $\times 86$ | 11000 | 222 | 13.00 | 209.0 | 20.6 | 94.7 | 8.53 | 92.8 | 31.4 | 300 | 53.4 |
| W200 $\times 71$ | 9100 | 216 | 10.20 | 206.0 | 17.4 | 76.6 | 769 | 9.7 | 25.4 | 247 | 52.8 |
| W $200 \times 50$ | 7580 | 210 | 9.14 | 205.0 | 14.2 | 61.2 | 583 | 89.9 | 20.4 | 199 | 51.9 |
| W200 $\times 46$ | 5890 | 203 | 72.4 | 2030 | 11.0 | 45.5 | 4.48 | 87.9 | 15.3 | 151 | 51.0 |
| W200 $\times 36$ | 4570 | 201 | 6.22 | 165.0 | 10.2 | 34.4 | 342 | 86.8 | 7.64 | 92.6 | 40.9 |
| W200 $\times 22$ | 2860 | 206 | 6.22 | 1020 | 8.0 | 20.6 | 19.4 | 83.6 | 1.42 | 27.8 | 22.3 |
| W150 $\times 37$ | 4730 | 162 | 8.13 | 1540 | 11.6 | 22.2 | 27. | 68.5 | 7.07 | 91. | 38.7 |
| W150 $\times 30$ | 3790 | 153 | 6.60 | 1530 | 03 | 17.1 | 2 B | 67.2 | 584 | 72.4 | 38.2 |
| W150 $\times 22$ | 2860 | 152 | 58.8 | 1520 | 6.6 | 12.1 | 159 | 65.0 | 3.87 | 509 | 36.8 |
| W150 $\times 24$ | 3060 | 160 | 6.60 | 102.0 | 10.3 | 13.4 | 168 | 66.2 | 1.83 | 35.9 | 24.5 |
| W: $50 \times 18$ | 2290 | 153 | 5.84 | 1020 | 7.1 | 9.19 | 120 | 63.3 | 1.26 | 24.7 | 23.5 |
| W150 $\times 14$ | 1730 | 150 | 4.32 | 10000 | 55 | 6.84 | 91.2 | 62.9 | 0.912 | 18.6 | 230 |

